

# **SECOND-ORDER OPTIMIZATION OF FRAMES WITH NONLINEAR BEHAVIOR**

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# PROBLEM

- Minimize the cost of a portal frame

# FRAME BEHAVIOR

- Linear elastic between plastic hinges
- Plastic hinges with limited plastic rotation
- Plastic hinges are automatically located in the most critical positions

# OPTIMIZATION APPROACH

- Solve of a nonlinear program
- Second-order approximation
- Integrated formulation
- No sensitivity analysis
- All the problem variables are present in the nonlinear program

# OPTIMIZATION SOFTWARE

- NEWTOP
- General purpose code
- Lagrange-Newton method
- Symbolic manipulation of all the functions


# NONLINEAR PROGRAMMING

Minimize  $f(\underline{x})$

subject to

$$\underline{g}(\underline{x}) \leq \underline{0} \quad \rightarrow \quad g_i(\underline{x}) + s_i^2 = 0$$

$$\underline{h}(\underline{x}) = \underline{0}$$

- Variables / functions  real and continuous
- All the functions are generalized polynomials, such as:

$$f(\underline{x}) = 5.9x_1^2x_4^{-3} - 3.1x_2 + 2.7x_1^{-1}x_3x_5^2 - 1.8$$

# GENERALIZED POLYNOMIALS

$$f(\underline{x}) = 5.9x_1^2x_4^{-3} - 3.1x_2 + 2.7x_1^{-1}x_3x_5^2 - 1.8$$

- A symbolic manipulation is performed
- Expression parsing and evaluation is simplified
- Exact first and second derivatives can be easily calculated
- All these operations can be efficiently performed

# INPUT FILE

```
### Main title of the nonlinear program
      Symmetric truss with two load cases (kN,cm)
Min.
      +565.685 * t5 ^ 2 + 100 * t8 ^2 ; # truss volume (cm3)

s.t.i.c.
      Min. area 4:   - t4 ^ 2 + 0.15 < 0 ;

s.t.e.c.
      Equil 16:   + 141.421 * t5 ^ 2 * disp16 - 100 = 0 ;

END_OF_FILE
```

- All the software is coded in ANSI C

# LAGRANGIAN

$$L(\underset{\sim}{X}) = f(\underset{\sim}{x}) + \sum_{k=1}^m \lambda_k^g \left[ g_k(\underset{\sim}{x}) + s_k^2 \right] + \sum_{k=1}^p \lambda_k^h h_k(\underset{\sim}{x})$$

# VARIABLES

$$\underset{\sim}{X} = \left( \underset{\sim}{s}, \underset{\sim}{\lambda}^g, \underset{\sim}{x}, \underset{\sim}{\lambda}^h \right)$$

# SOLUTION

- Stationary point of the Lagrangian



# SYSTEM OF NONLINEAR EQUATIONS

$$\nabla L(\tilde{X}) = \underset{\sim}{0} \Rightarrow \begin{cases} 2s_i \lambda_i^g = 0 & (i = 1, \dots, m) \\ g_i + s_i^2 = 0 & (i = 1, \dots, m) \\ \frac{\partial f}{\partial x_i} + \sum_{k=1}^m \lambda_k^g \frac{\partial g_k}{\partial x_i} + \sum_{k=1}^p \lambda_k^h \frac{\partial h_k}{\partial x_i} = 0 & (i = 1, \dots, n) \\ h_i = 0 & (i = 1, \dots, p) \end{cases}$$

- The solution of the system is a KKT solution when

$$\lambda_{\sim}^g \geq 0$$

# LAGRANGE-NEWTON METHOD

- The system of nonlinear equations

$$\nabla L(\tilde{X}) = \tilde{0}$$

is solved by the Newton method

- In each iteration the following system of linear equations has to be solved

$$H(\tilde{X}^{q-1}) \Delta \tilde{X}^q + \nabla L(\tilde{X}^{q-1}) = \tilde{0}$$

# HESSIAN MATRIX

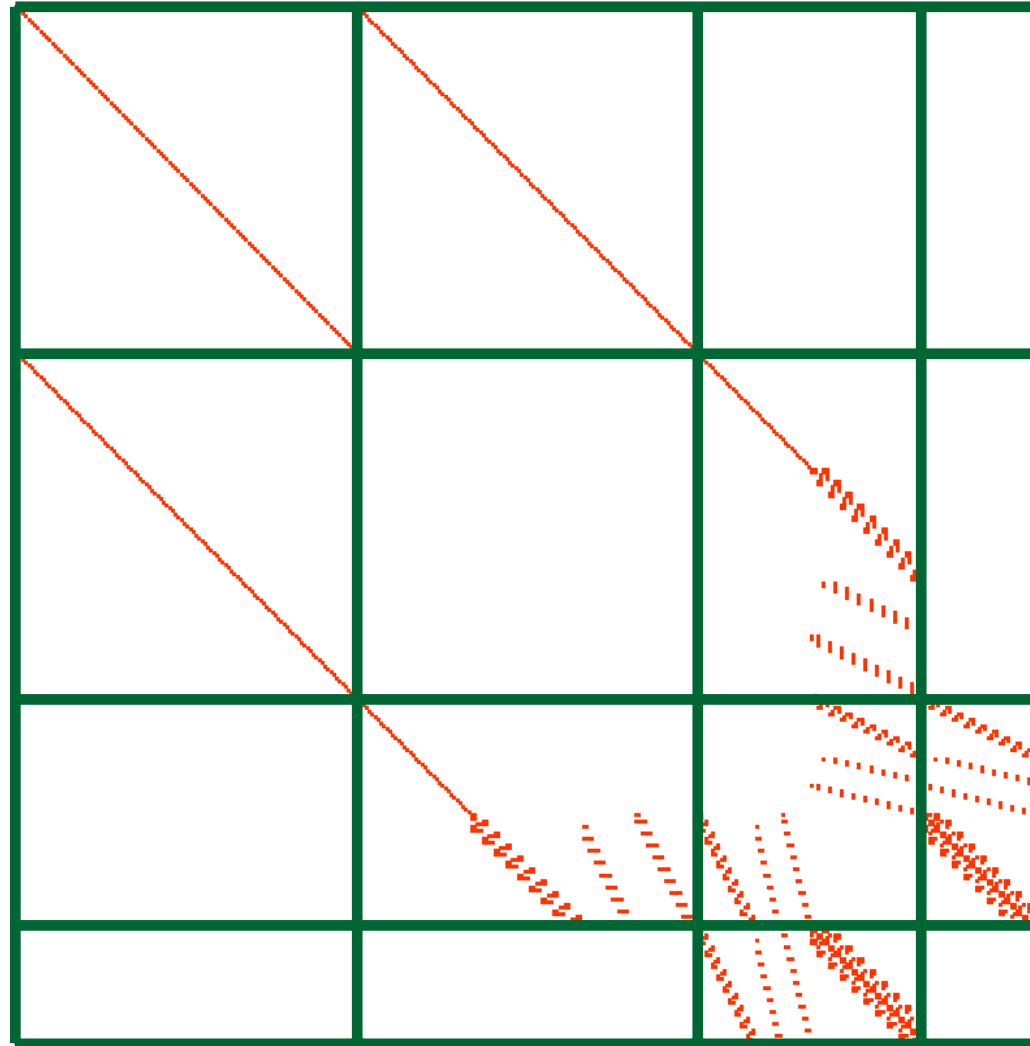
$\tilde{H} =$

	(m)	(m)	(n)	(p)
(m)	$Diag(2\lambda_i^g)$	$Diag(2s_i)$	$0$ ~	$0$ ~
(m)		$0$ ~	$\frac{\partial g_i}{\partial x_j}$	$0$ ~
(n)			$\bullet$	$\frac{\partial h_j}{\partial x_i}$
(p)	<i>SYMMETRIC</i>			$0$ ~

$\bullet \frac{\partial^2 f}{\partial x_i \partial x_j} + \sum_{k=1}^m \lambda_k^g \frac{\partial^2 g_k}{\partial x_i \partial x_j} + \sum_{k=1}^p \lambda_k^h \frac{\partial^2 h_k}{\partial x_i \partial x_j}$

# HESSIAN MATRIX SPARSITY PATTERN

$H =$   
 $\sim$



# SYSTEM OF LINEAR EQUATIONS

- Gaussian elimination
  - ◆ adapted to the sparsity pattern of the Hessian matrix
- Conjugate gradients
  - ◆ diagonal preconditioning
  - ◆ adapted to an indefinite Hessian matrix

## LINE SEARCH

$$\tilde{X}^q = \tilde{X}^{q-1} + \alpha \Delta \tilde{X}^q$$

# ***NEWTOP* COMPUTER CODE**

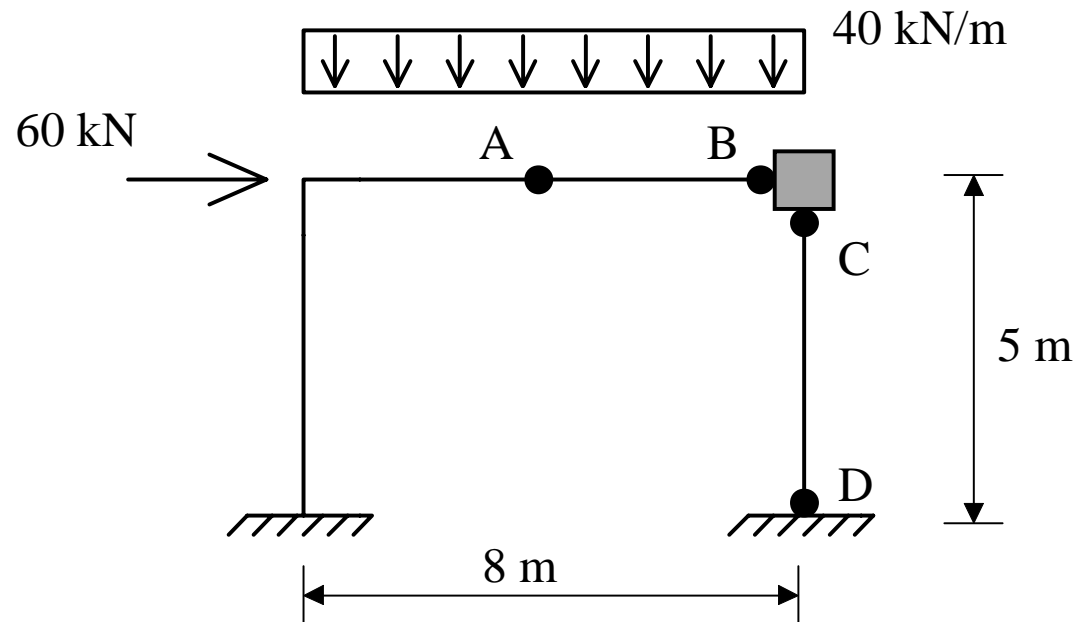
- All the variables are scaled
- Constraints are normalized
- Elementary equality constraints are substituted:

$$x_i = c x_j \quad \text{or} \quad x_i = c$$

- The NLP is simplified
- Large scale problems can be solved

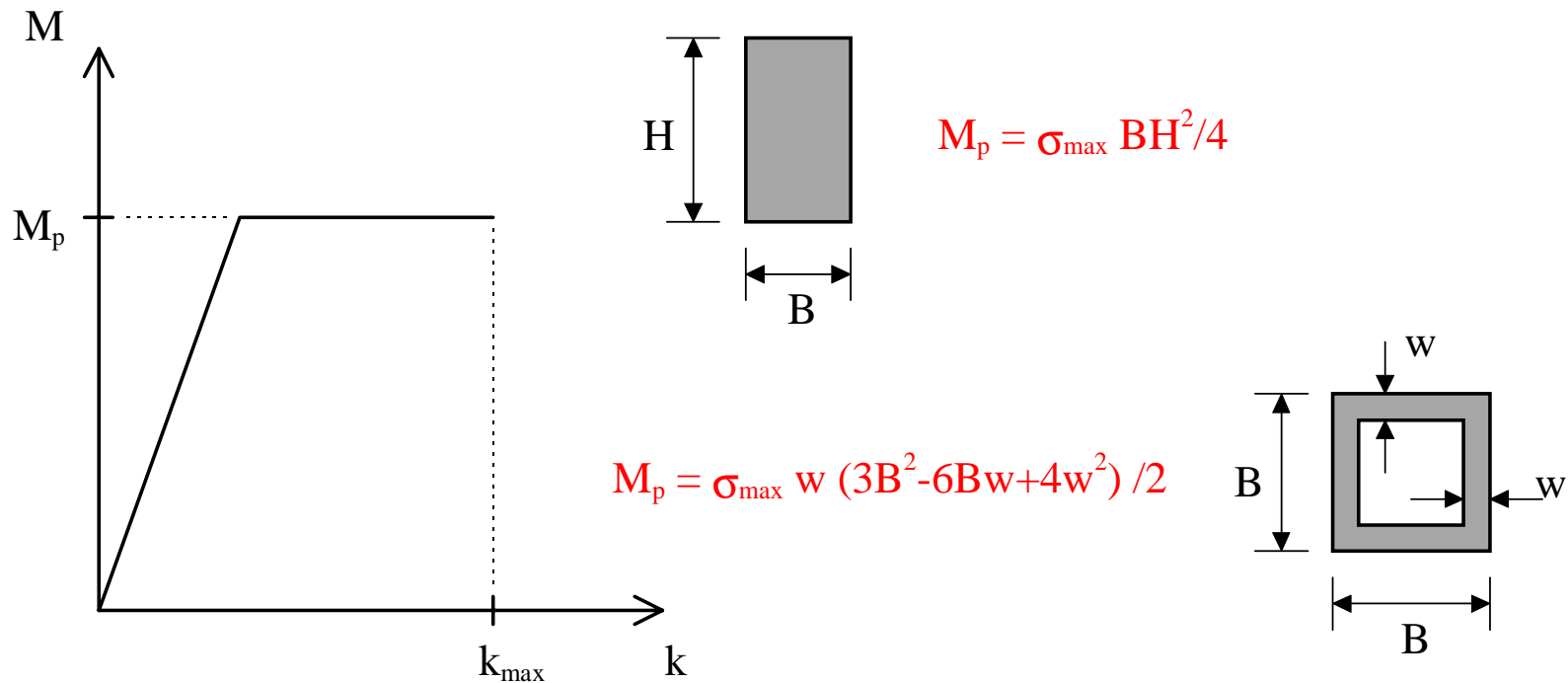
# PORTAL FRAME

- Cost minimization
- Independent design variables  $\Rightarrow$  cross section parameters



# NONLINEAR MATERIAL BEHAVIOR

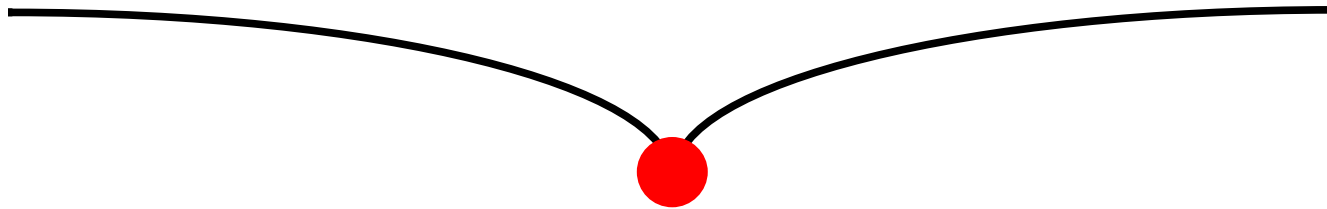
- Linear-perfectly plastic behavior
- Linear-constant moment-curvature diagram



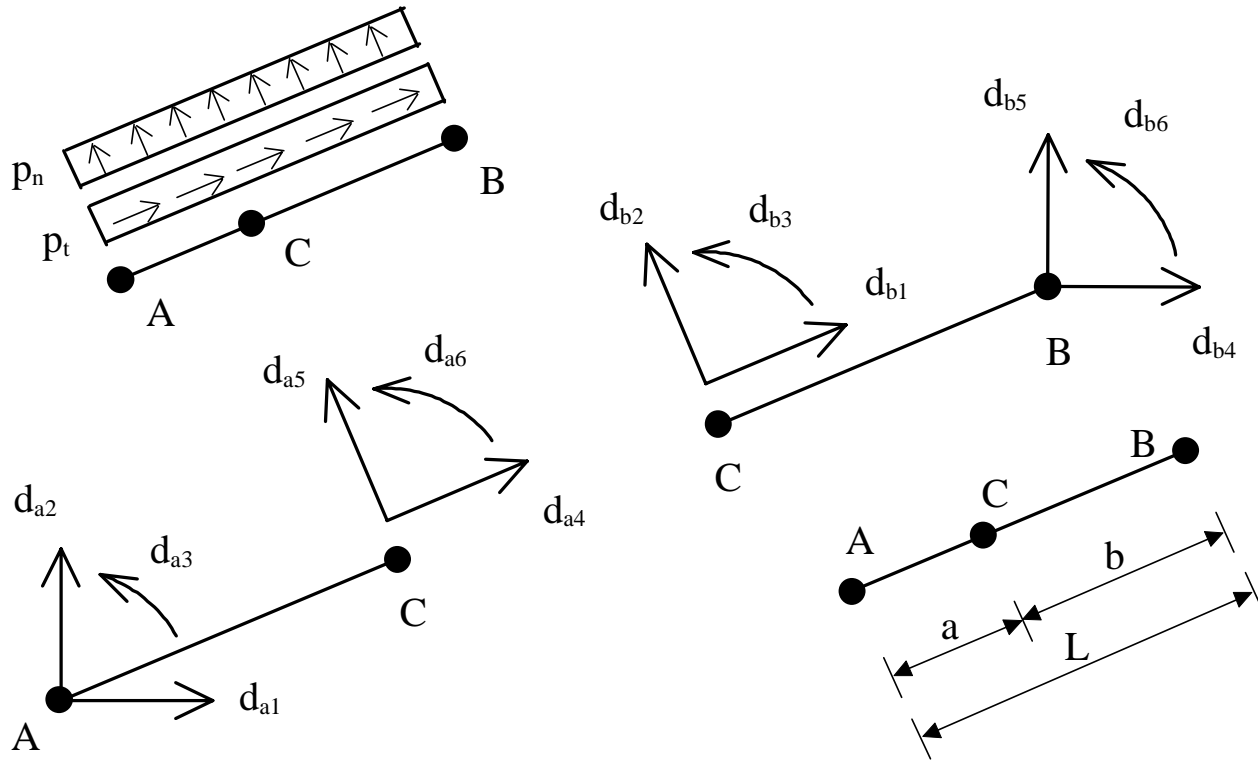


# NONLINEAR MATERIAL BEHAVIOR

- Plastic deformations concentrated in plastic hinges
- Plastic hinge rotation may be limited
- Collapse mechanism may not be reached
- Linear behavior between plastic hinges



# STRUCTURAL DISCRETIZATION

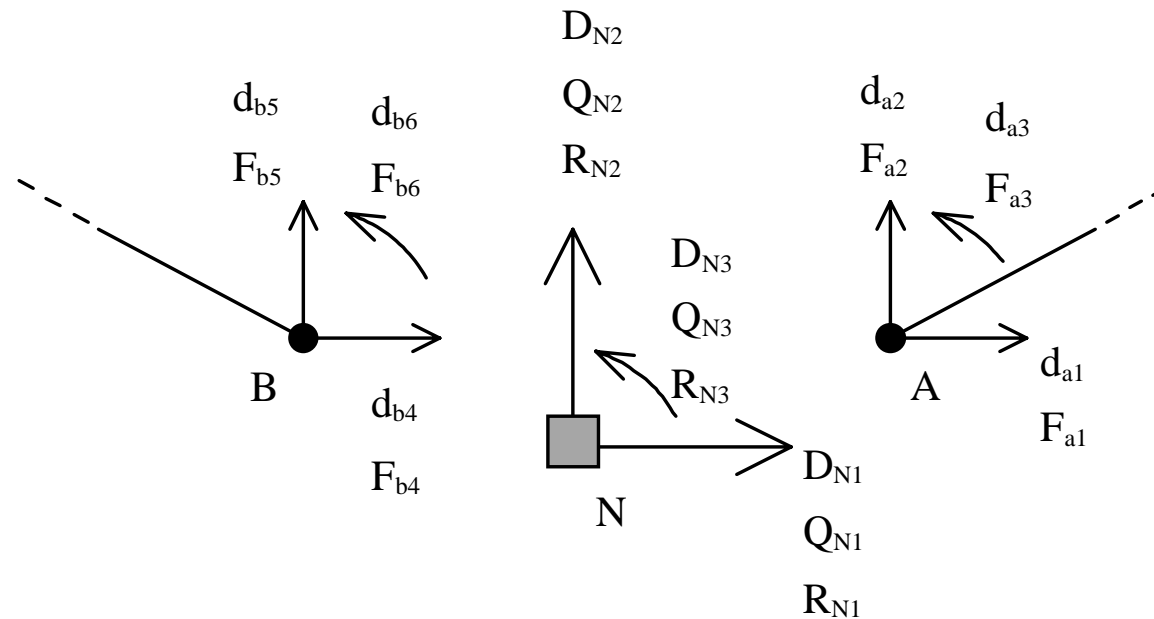


$$\tilde{K}_a \tilde{d}_a = \tilde{F}_a + \tilde{P}_a$$

$$\tilde{K}_b \tilde{d}_b = \tilde{F}_b + \tilde{P}_b$$

- Hinge C  $\Rightarrow$  point of max. bending moment

# EQUILIBRIUM EQUATIONS



$$\tilde{F}_{\tilde{a}}' + \dots + \tilde{F}_{\tilde{b}}'' + \dots = \tilde{Q} + \tilde{R}$$

- Reactions are only present in constrained dof 's

# NON LINEAR PROGRAM

- Objective function: cost  $\Rightarrow f(\tilde{x}) = \sum_{i=1}^{NB} c_i A_i L_i$
- Equality constraints:
  - ♦ beam behavior  $\Rightarrow K_{\tilde{a}} d_{\tilde{a}} = F_{\tilde{a}} + P_{\tilde{a}}$
  - ♦ equilibrium  $\Rightarrow F_{\tilde{a}}' + \dots + F_{\tilde{b}}'' + \dots = Q + R$
  - ♦ compatibility  $\Rightarrow d_{a1} = D_{N1}$
  - ♦ cross section properties  $\Rightarrow A, I, M_p = \dots$
  - ♦ beam length  $\Rightarrow L = a + b$

- Equality constraints (cont.):

- ◆ plastic hinge rotation  $\Rightarrow \theta_A = d_{a3} - D_{N3}$

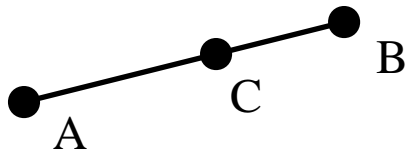
- ◆ elastic-plastic complementary in each plastic hinge:

$$\theta_A = 0 \text{ or } F_{a3} = M_p \text{ or } F_{a3} = -M_p \Rightarrow \theta_A (M_p^2 - F_{a3}^2) = 0$$

- ◆ nodal displacement  $\Rightarrow D_j = \bar{D}$  (only at prescribed dof 's)

- ◆ reaction  $\Rightarrow R_k = 0$  (only at non prescribed dof 's)

- ◆ null shear force in C  $\Rightarrow F_{a5} = 0$  (locates C in the point of max. bending moment)



- Inequality constraints:

- ◆ side constraints  $\Rightarrow x_{\min} \leq x_i \leq x_{\max}$

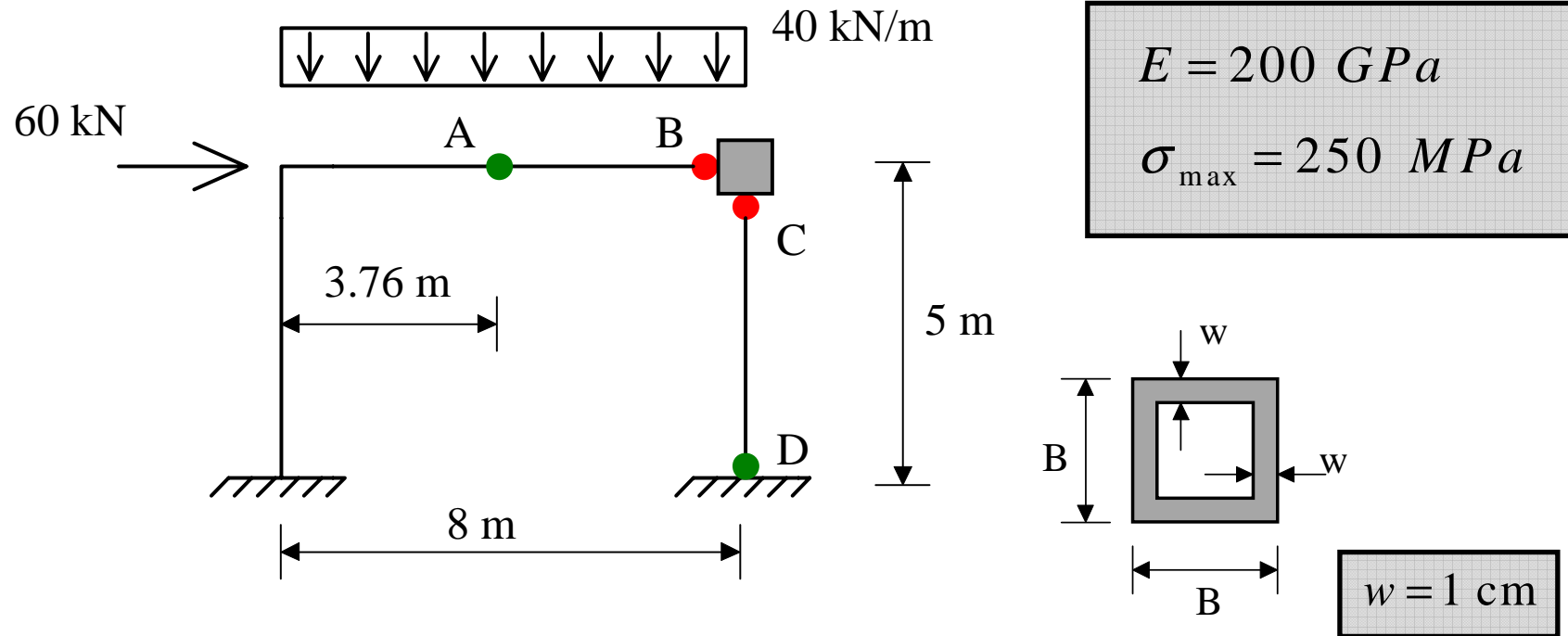
- ◆ moments are limited by  $\pm M_p \Rightarrow -M_p \leq F_{a3} \leq M_p$

- ◆ limited plastic hinge rotation  $\Rightarrow -\theta_{\min} \leq \theta_C \leq \theta_{\max}$

- limiting values depend on the type of material and on the shape of the cross section

- crushing, brittle failure and local buckling can thus be avoided

# NUMERICAL EXAMPLE



# NUMERICAL RESULTS

- Optimal solution - linear behavior
  - ◆ Volume = 0.175 m<sup>3</sup>
  - ◆ Horizontal displacement = 2.7 cm
- Optimal solution - nonlinear behavior
  - ◆ Volume = 0.157 m<sup>3</sup> (10 % smaller)
  - ◆ Horizontal displacement = 5.8 cm (2 x)



# CONCLUSIONS

- ↑ • More realistic approach of the frame design problem
- ↑ • Ultimate and serviceability conditions may be considered
- ↑ • More economical structures can be designed
- ↓ • Friendly user interface is still required
- ↓ • Solving the nonlinear program is still a *hard* task